ADDENDUM NO. 2
CONTRACT: RDSD 1602
DATED: JANUARY 25, 2017

TOWN OF RIVERHEAD
RIVERHEAD SEWER DISTRICT

PROHEALTHCARE SEWER CONNECTION

1. <u>DELIVERY VIA EMAIL CONFIRMATION TO ALL REGISTERED BIDDERS THROUGH E-NOTIFICATIONS ON TOWN OF RIVERHEAD WEBSITE</u>

This Addendum is hereby made a part of the Contract Documents for the above noted project and construction contract and modifies the Original Bidding Documents. The Bidder shall note the following clarifications, deletions, additions, and changes to the Contract Documents and shall submit his/her bid, and be otherwise governed accordingly. Failure to acknowledge the Addendum may subject the Bidder to disqualification.

<u>Bidder shall note the following</u>:

1. *Acknowledge receipt of the Addendum on page PB-7 located in the Contract Book of the Bid Documents.*

2. *Addendum No. 2 for Project No. RDSD 16-02 consists of 12 pages in total. Pages 1 – 3 of this addendum consist of written changes to the Project Manual. The remaining 9 pages of this Addendum consist of the Geotechnical Subsurface Report to be included as a document for information only at the end of the Project Manual.*

<u>PROJECT MANUAL</u>

A. <u>INFORMATION ONLY DOCUMENTS:</u>

1. INSERT THE ATTACHED GEOTECHNICAL SUBSURFACE REPORT AT THE END OF THE PROJECT MANUAL.

B. <u>PROPOSAL:</u>

1. ITEM 6, UTILITY RELOCATION ALLOWANCE: REVISE TO "**FIFTY TWO THOUSAND DOLLARS AND ZERO CENTS**" IN WORDS, AND "**$52,000.00**" IN NUMERALS ON PAGE PB-2.

C. <u>SECTION 012100 - ALLOWANCES:</u>

1. 1.06 F: DELETE WHAT IS WRITTEN AND INSERT "**INCLUDE THE CASH ALLOWANCE OF $52,000 (FIFTY TWO THOUSAND DOLLARS AND ZERO CENTS) IN THE AMOUNT BID FOR USE UPON THE OWNER'S INSTRUCTIONS.**"

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2. 1.06 G: DELETE WHAT IS WRITTEN AND INSERT "**FUNDS WILL BE DRAWN FROM THIS ACCOUNT TO PAY FOR POTENTIAL UTILITY RELOCATIONS AS SPECIFIED IN SECTION 014100. THE ACTUAL INVOICED CHARGES OF THE UTILITY COMPANY INCURRED IN CONNECTION WITH RELOCATING EXISTING UTILITIES AND THE INSTALLATION OF NEW UTILITIES SHALL BE PAID FOR OUT OF THIS CASH ALLOWANCE. THE MAINTENANCE AND PROTECTION OF TRAFFIC (MPT) AND FINAL RESTORATION OF SURFACES IN AREAS IMPACTED BY THE UTILITY RELOCATION WORK SHALL BE PROVIDED BY THE CONTRACTOR UNDER THEIR BASE BID WORK AND IS NOT ELIGIBLE FOR ADDITIONAL COMPENSATION AND/OR REIMBURSEMENT OUT OF THIS ALLOWANCE.**"

CONTRACT NO. RDSD 16-02

A: NBONO@H2M.COM | PM: | LSB: NICHOLAS F. BONO |
LSD: 1/25/2017 10:19 AM | LPD: 1/25/2017 10:30 AM | 20 JANUARY 2017| R#: 7 | P: 3 |
X:\RDSD (RIVERHEAD SEWER DISTRICT) - 10805\RDSD1602 PROHEALTHCARE SEWER DESIGN\03-BID TAB & CONTRACT ADDENDUMS\ADDENDUM NO.2\RDSD 1602 - CONTRACT ADDENDUM NO.2.DOC |
ADDENDUM NO. 2 | RDSD 1602 |

ADDENDUM NO. 2
CONTRACT: RDSD 1602
DATED: JANUARY 25, 2017

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ATTACHMENTS

GEOTECHNICAL SUBSURFACE REPORT

(FOR INFORMATION ONLY – NOT PART OF CONTRACT DOCUMENTS)

MUNICIPAL TESTING LABORATORY, INC.

375 RABRO DRIVE, HAUPPAUGE, NEW YORK, 11788 (631) 761-5555; FAX (631) 761-5560
A FULL SERVICE LABORATORY AND ENGINEERING CONSULTANT; STEEL, CONCRETE, SOILS, ASPHALT, ASBESTOS, & ALL BUILDING MATERIALS

January 24, 2017

GEOTECHNICAL SUBSURFACE REPORT

OF

Prohealthcare Medical Center
CR-58 and Ostrander Avenue
Riverhead, NY

FOR

H2M Architects + Engineers

538 Broad Hollow Road

Melville, N.Y.

01/24/2017

MUNICIPAL TESTING LABORATORY, INC.

375 RABRO DRIVE, HAUPPAUGE, NEW YORK, 11788 (631) 761-5555; FAX (631) 761-5560
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January 24, 2017

Mr. Nicholas F. Bono, P.E.
H2M Architects + Engineers
538 Broad Hollow Road
Melville, N.Y. 11747

RE: Porhealthcare Medical Center, Riverhead, NY
CR-58 and Ostrander Avenue
Geotechnical Investigation

Dear Mr. Bono:

I have examined the boring log from December 14th of 2016, reviewed the site conditions and note the following;

SCOPE:

The site is located at the intersection of CR-58 and Ostrander Avenue in Riverhead, NY. The existing site consists of an asphalt paved road. The purpose of the investigation is to evaluate the soil conditions for a sewer extension.

The services requested by H2M Architects + Engineers consisted of performing soil boring tests using a standard penetrant test (SPT) in accordance with the American Standard for Testing Materials (ASTM) D1586-08a "Standard Test Method for Penetration Test and Split-Barrel Sampling of Soils". A single boring was performed to a depth of fifteen (15) feet below existing grade (BG). The location of the boring is shown on the attached site map. Soil boring was drilled using 4 inch interior diameter direct push with a standard 2 inch split spoon for SPT test and sampling. The drilling was performed utilizing a geoprobe soil boring rig. The rig utilized an automatic 140 lb hammer with a 30 inch drop for SPT sampling.

January 24, 2017

GEOTECHNICAL INVESTIGATION:

The boring was located at the intersection of CR-58 and Ostrander Avenue. Good bearing strata material was found beginning at the ground surface below the existing pavement, consisting generally of coarse brown sand. This material classifies as a SW-SM type material per ASTM D2487-10 ("Standard Practice for Classification of Soils for Engineering Proposes (Unified Soil Classification System.

The SPT N value blow counts recorded ranged from six (6) to thirteen (13) overall. The New York State Building Code allows sandy soils a bearing capacity of one (1) tons per square foot (TSF). Based on the results of the SPT tests the following geotechnical parameters can be derived empirically from various engineering sources:

Parameter	B1
Soil Bearing Capacity	0.6 to 1.3 TSF
Subgrade Modulus	200-300 psi/in
Angel of Internal Friction	30-35
Soil Unit Weight	90-115 pcf
Bearing Capacity Factors	
Nc	35
Nq	23
Ny	22

January 24, 2017

DEWATERING

Based on the borings drilled, groundwater was encountered at sixteen (16) feet during boring operations. Groundwater be of concern during construction depending on the depth of the installation. Note, groundwater determinations are affected by specific sampling time, long periods of precipitation, heavier than normal precipitation, higher than normal tidal events, temperature, and other factors. As such these readings are considered as a general range and shall not be considered as absolute.

EXCAVATION FOR STRUCTURES:

If a spread footing foundation system is to be used, the footing subgrade must be inspected by a qualified professional engineer. All excavated areas will be recompacted to a maximum density of 95% of a modified proctor ("ASTM D1557-09 Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft^3(2,700 kN-m/m^3)"), the following shall be considered:

- Excavate to indicated design elevations and dimensions within a tolerance of plus or minus 6 inches. Extend excavations a sufficient distance from structures for placing and removing concrete formwork, for installing services and other construction, and for inspections.
- Remove all (if found) unsuitable fill material to undisturbed soil and remove organic material beneath the new building foundation.
- Remove (if found) material containing clays and expansive soils, construction debris, cobbles and boulders.
- Replace all unsuitable soil (if found) in accordance with the recommendations in the next section for compaction of backfills and fills.

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January 24, 2017

COMPACTION OF BACKFILLS AND FILLS:

The material to be used as backfill shall be of SW, SP, SM, GW, GP, GM type in accordance with ASTM D2487-10 ("Standard Practice for Classification of Soils for Engineering Proposes (Unified Soil Classification System)"). Material used for backfilling shall be free of organic material, construction debris, cobbles and boulders. Backfill materials shall be placed in lifts not more than 6 inches in loose depth for material compacted by hand operated tampers, ten (10) inches in loose depth for compaction rollers of at least a 1,000 lbs capacity.

Field compaction shall be confirmed in accordance with the requirements of ASTM D6938-10 ("Standard Test Method for In-Place Density and Water Content of Soil and Soil-Aggregate by Nuclear Methods (Shallow Depth)"). At minimum, perform field testing at one test per every 2000 square feet of material placed per lift and at least once every lift until completed. Compact soil materials to not less than the 95% of a maximum dry unit weight according to ASTM D1557-09 (ASTM D1557-09 Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft^3(2,700 kN-m/m^3)).

January 24, 2017

RECOMMENDATIONS/CONCLUSIONS:

Based on the New York State Building Code, site conditions and the geotechnical testing performed, the presumptive bearing capacity is 1 TSF. Based on the SPT results the soil has a bearing capacity ranging from 0.6 TSF to 1.3 TSF.

The Site is an improved land and as such may contained foreign materials that were not otherwise noted in this report. The extent of the investigation is only for the exact locations sampled, no other assumptions are made as to other subsurface conditions.

Based on the above, a qualified Geotechnical Engineer shall be retained during the construction of the foundations and to determine if any conditions exist that effect the above stated recommendations, as well as to ensure that specified compaction bearing is achieved.

Respectfully Submitted,

Stephen M. Latus, P.E.

MUNICIPAL TESTING LABORATORY, INC.

AN ENGINEERING INSPECTION AND TESTING COMPANY

375 RABRO DRIVE, HAUPPAUGE, NY 11788 - PH: (631)761-5555; FAX (631)761-5560

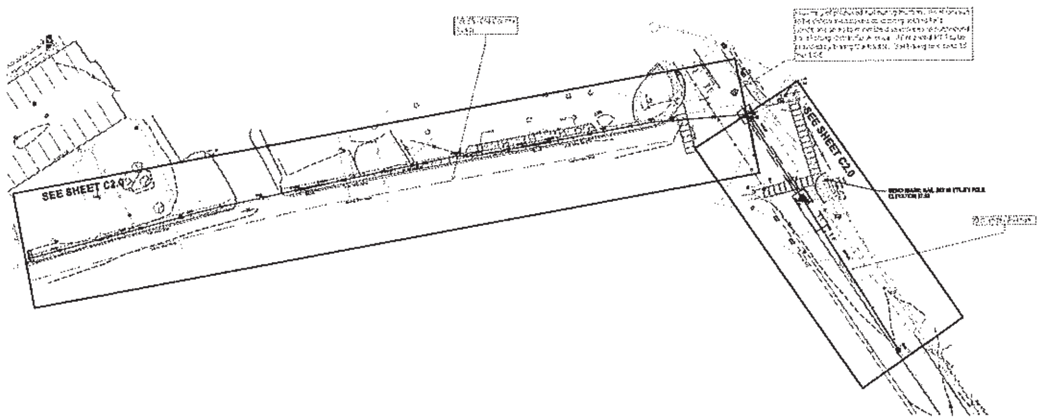
CLIENT:	H2M Architects + Engineers	BORING #:	1
PROJECT:	Prohealthcare	PERCOLATION TEST #:	
	Old Country Rd. and Ostrander Ave.,	DEPTH OF HOLE:	16'
	Riverhead, NY	ROCK CORE:	
DATE DRILLED:	12/14/16	ROCK RECKOVERY:	
DRILLER:	Dave Johnson		
HELPER:		SURFACE ELEVATION:	
TYPE OF RIG:	GEOPROBE	GROUND WATER TABLE:	NA

	TIME	WATER DROP
	30 SEC	
	1.00 MIN	
	5.00 MIN	
	10.00 MIN	
	15.00 MIN	
	30.00 MIN	

DEPTH BELOW SURFACE	SAMPLE NUMBER	BLOWS PER 6" ON SAMPLER		CASING BLOWS PER FT.	SOIL SAMPLE DESCRIPTION	BEARING CAPACITY	DRILLING REMARKS
	S1	5	5		22" CBS		GPR USED ONSITE
		5	5		SW-SM	1 TSF	CBS-Coarse Brown Sand
	S2	3	3		21" CBS		
		3	3		SW-SM	0.6 TSF	
5	S3	4	4		21" CBS		
		3	3		SW-SM	0.7 TSF	
	S4	5	4		23" CBS		
		5	5		SW-SM	0.9 TSF	
	S5	8	7		16" CBS		
10		6	7		SW-SM	1.3 TSF	
	S6	9	7		20" CBS		
		6	7		SW-SM	1.3 TSF	
	S7	8	6		20" CBS		
		6	5		SW-SM	1.2 TSF	
15	S8	5	4		20" CBS		
		6	7		SW-SM	1 TSF	
20							
25							
30							
35							

STEPHEN M. LATUS, PE

SEE BORING LOCATION PLAN IN THE FOLLOWING PAGE



① OVERVIEW PLAN

GENERAL NOTES

UTILITY NOTES

LEGEND

DRAINAGE & SANITARY STRUCTURE SCHEDULE						
STRUCT. NUMBER	STRUCTURE TYPE	FRAME ELEVATION (FEET)	TOP OF RING ELEVATION (FEET)	BOTTOM OF STRUCTURE ELEVATION (FEET)	EFFECTIVE DEPTH (FEET)	INVERT ELEVATIONS (FEET)
[illegible]	SANITARY MANHOLE, [illegible] CONCRETE WITH SLAB	[illegible]	NA	[illegible]	NA	[illegible]
[illegible]	CATCH BASIN, [illegible] CONCRETE	[illegible]	NA	[illegible]	NA	NO PIPES PRESENT
[illegible]	ROUND INLET, [illegible]	[illegible]	NA	[illegible]	NA	[illegible]
[illegible]	CATCH BASIN, [illegible] CONCRETE	[illegible]	NA	[illegible]	NA	NO PIPES PRESENT
[illegible]	ROUND INLET	[illegible]	NA	[illegible]	NA	NO PIPES VISIBLE

H2M architects + engineers

MARK	DATE	DESCRIPTION

RIVERHEAD SEWER DISTRICT

PROHEALTHCARE SEWER CONNECTION

RIVERHEAD, NY

CONTRACT 8

REGULATORY REVIEW

OVERVIEW PLAN

C1.0